The Titen HD<sup>®</sup> anchor is a patented, high-strength screw anchor for concrete and masonry. It is designed for optimum performance in both cracked and uncracked concrete; a requirement that the 2009 IBC places on post-installed anchors. The high strength, easy to install Titen HD anchor has been tested and shown to provide outstanding performance in cracked and uncracked concrete under both static and seismic loading conditions. The self-undercutting, non-expansion characteristics of the Titen HD anchor make it ideal for structural applications, even at reduced edge distances and spacings. Recommended for permanent dry, interior non-corrosive environments or temporary outdoor applications.

#### PERFORMANCE FEATURES:

- · Tested per AC193 to ensure outstanding performance in both cracked and uncracked concrete
- Higher load capacity and vibration resistance: Threads along the length of the anchor undercut the concrete and efficiently transfer the load to the base material.
- Vibration and shock resistance: The mechanical interlock of the threads and the ratchet teeth on the underside of the head help prevent the anchor from loosening in vibratory conditions. The Titen HD anchor has been tested to 12.6 million vibratory cycles with no performance reductions.
- Specialized heat treating process: Creates superior surface hardness at the tip to facilitate cutting, while at the same time not compromising ductility within the anchor body.
- Less spacing and edge distance required: The anchor does not exert expansion forces on the base material.
- Easy post-installation inspection: The head is stamped with the Simpson Strong-Tie<sup>®</sup> "≠" sign and the anchor length in inches.

#### **INSTALLATION FEATURES:**

**Mechanical Anchors** 

- · No special drill bit needed: Designed to install using standard sized ANSI tolerance drill bits
- Installs with 50% less torque: Testing shows that when compared to competitors, the Titen
- HD requires 50% less torque to be installed in concrete.
- Hex-washer head: Requires no separate washer and provides a clean installed appearance.\*
- Removable: Ideal for temporary anchoring (e.g. formwork, bracing) or applications where fixtures
  may need to be moved. Re-use of the anchor to achieve listed load values is not recommended.
  See reinstallation note on next page.

MATERIAL: Carbon steel, heat treated

#### FINISH: Zinc plated or mechanically galvanized

**CODES:** ICC-ES ESR-2713 (concrete); ICC-ES ESR-1056 (CMU); City of L.A. RR25741(concrete)City of L.A. RR25560(CMU); Florida FL 11506.7; Factory Mutual 3017082, 3035761 and 3043442. The load tables list values based upon results from the most recent testing and may not reflect those in current code reports. Where code jurisdictions apply, consult the current reports for applicable load values.

**TEST CRITERIA:** The Titen HD<sup> $\odot$ </sup> anchor has been tested in accordance with ICC-ES AC193, ACI 355.2 and ICC-ES AC106 for the following:

- Static tension and shear loading in cracked and uncracked concrete
- Seismic and wind loading in cracked and uncracked concrete
- Performance in uncracked masonry

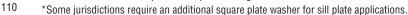
Anchor Fatigue Testing: Tested in accordance with ASTM E 488 for the effects of fatigue. 25% of the average ultimate load was applied to the anchor for 2 million cycles at a frequency of 15 Hz. Subsequent load tests showed no reduction in ultimate tension capacity.

Vibratory Load Testing: A 150 lb. concrete block was suspended from a %" diameter anchor embedded at 1 ½" and vibrated for 12.6 million cycles at a frequency of 30 Hz and an amplitude of 0.0325 inches. Subsequent load test showed no reduction in ultimate tension capacity. Field Testing: For guidance on field testing see technical bulletin T-SAS-THDINSP.

**INSTALLATION:** Holes in metal fixtures to be mounted should match the diameter specified in the table on the next page.

- Caution: Oversized holes in the base material will reduce or eliminate the mechanical interlock of the threads with the base material and will reduce the anchor's load capacity. Use a Titen HD screw anchor one time only. Installing the anchor multiple times may result in excessive thread wear and reduce load capacity.
  - Drill a hole in the base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed. Drill the hole to the specified embedment depth plus 1/2" minimum to allow the thread tapping dust to settle and blow it clean using compressed air. Overhead installations need not be blown clean. Alternatively, drill the hole deep enough to accommodate embedment depth and dust from drilling and tapping.
  - Insert the anchor through the fixture and into the hole.
  - Tighten the anchor into the base material until the hex washer head contacts the fixture.
  - Do not use impact wrenches to install into hollow CMU.

**SUGGESTED SPECIFICATIONS:** Screw anchors shall have 360-degree contact with the base material and shall not require oversized holes for installation. Fasteners shall be manufactured from carbon steel, and are heat-treated. Anchors shall be zinc plated in accordance with ASTM B633 or mechanically galvanized in accordance with ASTM B695. Anchors are not to be reused after initial installation. Screw anchors shall be Titen HD® anchors from Simpson Strong-Tie, Pleasanton, CA. Anchors shall be installed per the Simpson Strong-Tie instructions for the Titen HD anchor.









Strong





Screw Anchor U.S. Patent 5,674,035 & 6,623,228

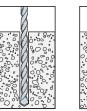
Titen HD®

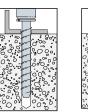


Suitable for use in place of code anchor bolts.

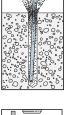


Longer ½" diameter Titen HD anchors achieve sufficient embedment depth to develop tension loads equal to many Simpson Strong-Tie holdowns that specify a %" diameter anchor. Testing has been conducted to assure compatibility of these holdowns' anchor holes with the ½" Titen HD screw anchor.





Installation Sequence





Su in a



### Titen HD® Anchor Product Data - Zinc Plated

Size	Model	Drill Bit Dia.	Wrench Size	Qua	ntity
(in.)	No.	(in.)	(in.)	Box	Carton
<b>¾</b> x 3	THD37300H			50	200
<b>¾</b> x 4	THD37400H	3/	9⁄16	50	200
<b>¾</b> x5	THD37500H	3⁄8	716	50	100
<b>¾</b> x6	THD37600H			50	100
¹∕₂ x 3	THD50300H			25	100
1⁄2 X 4	THD50400H			20	80
1∕₂ x 5	THD50500H			20	80
½x6	THD50600H			20	80
1⁄2 X 6 1⁄2	THD50612H	1/	3/.	20	40
1⁄2 X 8	THD50800H	1⁄2	3⁄4	20	40
½ x 12	THD501200H	]		20	40
½ x 13	THD501300H	]		20	40
½ x 14	THD501400H	]		20	40
½ x 15	THD501500H			20	40
5∕8 x 4	THD62400H			10	40
<b>5∕8</b> x 5	THD62500H			10	40
5%ax6	THD62600H	5⁄8	15/16	10	40
5% x 6 1∕2	THD62612H	1		10	40
5∕8 X 8	THD62800H	1		10	20
<b>5∕8</b> x 4	THDB62400H			10	40
<b>5∕8</b> X 5	THDB62500H	1		10	40
<b>5%</b> x 6	THDB62600H	5⁄8	<sup>15</sup> /16	10	40
5% x 6 1∕2	THDB62612H	1		10	40
5∕8 X 8	THDB62800H	1		10	20
3∕4 x 4	THD75400H			10	40
³⁄₄ x 5	THD75500H			5	20
¾ X 6	THDT75600H	3⁄4	+ 1/	5	20
<b>¾</b> X 7	THD75700H	9⁄4	1 1/8	5	10
3⁄4 X 8 1⁄2	THD75812H			5	10
¾ x 10	THD75100H			5	10

1. Zinc plating meets ASTM B633, SC1.

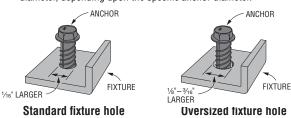
2. Length is measured from the underside of the head to the tip of the anchor.



### FIXTURE HOLE DIAMETER:

Due to the full shank diameter and larger threads of the Titen HD<sup>®</sup> screw anchor, consideration needs to be given to specifying the appropriate diameter Titen HD anchor based on the fixture hole type to be used. The American Institute of Steel Construction (AISC) has established the following guidelines with regards to fixture hole sizing depending on the hole type:

- "Standard" fixture holes are 1/16" larger than the nominal anchor diameter.
- "Oversized" fixture holes are 1/6 -3/6" larger than the nominal anchor diameter, depending upon the specific anchor diameter.



a the following table to identify which

Use the following table to identify which diameter Titen HD<sup>®</sup> screw anchor to use based on the fixture hole type and diameter. In most cases where a smaller diameter Titen HD anchor is called out in comparison to the competitor's larger diameter anchor, the Titen HD anchor still generally provides allowable tension and shear load values comparable to or greater than those of the competitor's anchor.

Titen HD Anchor Product Data	- Mechanically	Galvanized
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Size	Model	Drill Bit Dia.	Wrench Size	Qua	ntity
(in.)	No.	(in.)	(in.)	Box	Carton
<b>¾</b> x 5	THD37500HMG	3/8	<sup>9</sup> ⁄16	50	100
<b>¾</b> x6	THD37600HMG	78	716	50	100
1∕2 x 5	THD50500HMG			20	80
¹∕₂ x 6	THD50600HMG	1/2	3⁄4	20	80
1/2 x 6 1/2	THD50612HMG	72	9⁄4	20	40
1∕2 x 8	THD50800HMG			20	40
5∕8 x 5	THD62500HMG			10	40
5%sx6	THD62600HMG	5/8	15/16	10	40
5%s x 6 1∕2	THD62612HMG	78	'716	10	40
5∕8 X 8	THD62800HMG			10	20
<b>5∕8</b> x 5	THDB62500HMG			10	40
<b>5∕8</b> x 6	THDB62600HMG	5/	15/	10	40
5%s x 6 1∕2	THDB62612HMG	5⁄8	<sup>15</sup> ⁄16	10	40
5∕8 X 8	THDB62800HMG			10	20
3⁄4 x 8 1⁄2	THD75812HMG	3/.	114	5	10
¾ x 10	THD75100HMG	3⁄4	1 1⁄8	5	10

 Mechanical galvanizing meets ASTM B695, Class 65, Type 1. Intended for some pressure-treated wood sill plate applications. Not for use in other corrosive or outdoor environments. See page 11 or visit www.strongtie.com/info for more corrosion information.



The Titen HD<sup>®</sup> screw anchor  $\frac{3}{4}$  x 6" and  $\frac{3}{4}$  x 7" (models THDT75600H and THD75700H) have a 1" section under the head that is unthreaded to allow installation into tilt-up wall braces.



### **Hole Dimensions**

Titen HD Diameter (in.)	Wrench Size (in.)	Recommended Fixture Hole Size (in.)
3⁄8	<sup>9</sup> ⁄16	1⁄2 to 9⁄16
1⁄2	3⁄4	5⁄8 to 11⁄16
5⁄8	<sup>15</sup> ⁄16	3∕4 to <sup>13</sup> ∕16
3⁄4	1 1⁄8	7∕8 to <sup>15</sup> ∕16





### Titen HD® Anchor Installation Information and Additional Data<sup>1</sup>

Observation 1.11	0				Nomin	al Ancho	r Diamet	er (in.)		
Characteristic	Symbol	Units	:	3/8	1,	/2	l	<b>8</b> <sup>4</sup>	3	4
	Installa	ation Info	rmation							
Drill Bit Diameter	d <sub>bit</sub>	in.	1	3⁄8	1,	2	5/8 3/4		4	
Baseplate Clearance Hole Diameter	d <sub>c</sub>	in.	1,	2	5	8	8	/4	7	8
Maximum Installation Torque	Tinst, max	ft-lb	5	0 <sup>2</sup>	6	5 <sup>2</sup>	1(	)0 <sup>2</sup>	15	50 <sup>2</sup>
Maximum Impact Wrench Torque Rating	T <sub>impact,max</sub>	ft-lb	15	0 <sup>3</sup>	34	0 <sup>3</sup>	<sup>3</sup> 340 <sup>3</sup> 385 <sup>3</sup>			
Embedment Depth	h <sub>nom</sub>	in.	21⁄2	31⁄4	4 31/4 4			51⁄2	51⁄2	61⁄4
Critical Edge Distance	Cac	in.	2 11/16	3 %	3%16	4½	4½	6%	6 <b>%</b>	7 5⁄16
Minimum Edge Distance	C <sub>min</sub>	in.				1	3⁄4			
Minimum Spacing	Smin	in.	3							
Minimum Concrete Thickness	h <sub>min</sub>	in.	33/4	5	5	6¼	6	81⁄2	8¾	10
	Ad	ditional D	ata							
Anchor Category	category	—					1			
Yield Strength	f <sub>ya</sub>	psi				97,	000			
Tensile Strength	f <sub>uta</sub>	psi				110	,000			
Minimum Tensile & Shear Stress Area	Ase	in <sup>2</sup>	n <sup>2</sup> 0.099 0.183 0.276 0.414							14
Axial Stiffness in Service Load Range - Uncracked Concrete	$eta_{uncr}$	lb/in.				715	,000			
Axial Stiffness in Service Load Range - Cracked Concrete	$\beta_{cr}$	lb/in.				345	,000			

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318 Appendix D.

 T<sub>impact,max</sub> is the maximum permitted torque rating for impact wrenches for the embedment depth range covered by this table. This is not applicable to other embedment depths published elsewhere in this catalog.

2. T<sub>inst,max</sub> is the maximum permitted installation torque for the embedment depth range covered by this table for installations using a torque wrench. This is not applicable to other embedment depths published elsewhere in this

4. Data for 5%" anchor is only valid for THDB62 series.

catalog.



Titen HD <sup>®</sup> Anchor Tension Strength Design Data <sup>1</sup>						page 13 for le load table	an explana icons	tion		<b>.</b>
Characteristic	Cumhal	Unite	Nominal Anchor Diameter (in.)							
Gilaracteristic	Symbol	Units	1	%	1,	/2	5	<b>8</b> 9	3	4
Embedment Depth	h <sub>nom</sub>	in.	21⁄2	31⁄4	31⁄4	4	4	5½	5½	6¼
	Steel	Strength	in Tensio	ı						
Tension Resistance of SteelNsaIb.10,890						130	30,	360	45,	540
Strength Reduction Factor - Steel Failure $\phi$ — 0.65 <sup>2</sup>										
Concrete Bre	akout Stre	ngth in T	ension <sup>6,8</sup>							
Effective Embedment Depth	hef	in.	1.77	2.40	2.35	2.99	2.97	4.24	4.22	4.86
Critical Edge Distance <sup>6</sup>	Cac	Cin.	2 11/16	35⁄8	3%16	41⁄2	41⁄2	6 <b>¾</b>	6 <b>¾</b>	7 5⁄16
Effectiveness Factor - Uncracked Concrete	<i>k</i> uncr	-				2	4			
Effectiveness Factor - Cracked Concrete	kcr	-				1	7			
Modification Factor	$\Psi_{c,N}$	-				1	.0			
Strength Reduction Factor - Concrete Breakout Failure	φ	_				0.0	65 <sup>7</sup>			
	Pullout	Strength	in Tensio	n <sup>8</sup>						
Pullout Resistance, Uncracked Concrete (f'c=2,500 psi)	N <sub>p,uncr</sub>	lb.	2,700 <sup>4</sup>	3	3	3	3	9810	3	3
Pullout Resistance, Cracked Concrete (f'c=2,500 psi)	N <sub>p,cr</sub>	lb.	1,2354	2,700 <sup>4</sup>	3	3	3260 <sup>4</sup>	5570 <sup>4</sup>	6,070 <sup>4</sup>	7,1954
Strength Reduction Factor - Pullout Failure	φ	_				0.0	65 <sup>5</sup>			
Breakout or Pul	lout Stren	gth in Ten	sion for S	eismic Ap	plication	S <sup>8</sup>				
Nominal Pullout Strength for Seismic Loads (f'c=2,500 psi)	N <sub>p,eq</sub>	lb.	1,2354	2,7004	3	3	32604	5570 <sup>4</sup>	6,070 <sup>4</sup>	7,1954
Strength Reduction Factor - Breakout or Pullout Failure	φ	_				0.0	65 <sup>5</sup>			

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318 Appendix D, except as modified below.

The value of  $\varphi$  applies when the load combinations of ACI 318 Section 9.2 2. are used. If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\boldsymbol{\varphi}.$  Anchors are considered brittle steel elements.

Pullout strength is not reported since concrete breakout controls. 3.

Adjust the characteristic pullout resistance for other concrete compressive 4. strengths by multiplying the tabular value by (f'c,specified / 2,500)<sup>0.5</sup>

The value of  $\boldsymbol{\varphi}$  applies when both the load combinations of ACI 318 Section 5. 9.2 are used and the requirements of Section D4.4(c) for Condition B are met. If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\phi$ .

The modification factor  $\psi_{\text{cp},\text{N}}$  = 1.0 for cracked concrete. Otherwise, 6. the modification factor for uncracked concrete without supplementary reinforcement to control splitting is either: (1)  $\psi_{\text{cp},\text{N}}$  = 1.0 if  $c_{a,\text{min}} \geq c_{ac}$ 

or (2)  $\psi_{\text{cp,N}} = \frac{C_{a,\text{min}}}{C_{ac}} \ge \frac{1.5h_{ef}}{C_{ac}}$  if  $c_{a,\text{min}} < c_{ac}$ . The modification factor,

 $\psi_{\text{cp},\text{N}}$  is applied to the nominal concrete breakout strength,  $N_{\text{cb}}$  or  $N_{\text{cbg.}}$ 

7. The value of  $\phi$  applies when both the load combinations of ACI 318 Section 9.2 are used and the requirements of Section D4.4(c) for Condition B are met. If the load combinations of ACI 318 Section 9.2 are used and the requirements of Section D4.4(c) for Condition A are met, refer to Section D4.4 to determine the appropriate value of  $\varphi.$  If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\phi$ .

8. For sand-lightweight concrete, in lieu of ACI 318 Section D.3.4, modify the value of concrete breakout strength,  $N_{p,cr}$ ,  $N_{p,uncr}$  and  $N_{eq}$  by 0.6. All-lightweight concrete is beyond the scope of this table.

9. Data for 5%" anchor is only valid for THDB62 series.

### Simpson Strong-Tie<sup>®</sup> Anchoring and Fastening Systems for Concrete and Masonry

### Titen HD<sup>®</sup> Heavy Duty Screw Anchor for Cracked and Uncracked Concrete



Titen HD<sup>®</sup> Ar

en HD® Anchor Shear Strength Design	n Data <sup>1</sup>								w expla	bage 13 for an Ination of the table icons		
Ohenesteristis	Ormahal		Nominal Anchor Diameter (in.)									
Characteristic	Symbol	Units	3	8	1,	2	5/1	5	3	4		
Embedment Depth	h <sub>nom</sub>	in.	21⁄2	31⁄4	31⁄4	4	4	51⁄2	51⁄2	61⁄4		
	Ste	eel Strength in	Shear									
Shear Resistance of Steel	V <sub>sa</sub>	lb.	4,4	60	7,4	55	10,	000	16,	840		
Strength Reduction Factor - Steel Failure	φ	_				0.0	50 <sup>2</sup>					
		Concrete Bre	akout Stre	ngth in She	ear <sup>6</sup>							
Outside Diameter	do	in.	0.3	375	0.5	00	0.6	625	0.7	'50		
Load Bearing Length of Anchor in Shear	le	in.	1.77	2.40	2.35	2.99	2.97	4.24	4.22	4.86		
Strength Reduction Factor - Concrete Breakout Failure	φ	—				0.7	70 <sup>3</sup>					
		Concrete P	ryout Stren	gth in She	ar							
Coefficient for Pryout Strength	К <sub>ср</sub>	—		1.0				2.0				
Strength Reduction Factor - Concrete Pryout Failure	φ	_				0.	704					

	Stee	l Strength in S	hear for Seismic App	lications		
Shear Resistance of Single Anchor for Seismic Loads	V <sub>eq</sub>	lb.	2,855	4,790	8,000	9,350
Strength Reduction Factor - Steel Failure	φ			0.6	50 <sup>2</sup>	

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318 Appendix D. except as modified below.

2 The value of  $\phi$  applies when the load combinations of ACI 318 Section 9.2 are used. If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\phi$ . Anchors are considered brittle steel elements

3. The value of  $\phi$  applies when both the load combinations of ACI 318 Section 9.2 are used and the requirements of Section D4.4(c) for Condition B are met. If the load combinations of ACI 318 Section 9.2 are used and the requirements of Section D4.4(c) for Condition A are met, refer to Section D4.4 to determine the appropriate value of φ. If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\phi$ .

4. The value of  $\phi$  applies when both the load combinations of ACI 318 Section 9.2 are used and the requirements of Section D4.4(c) for Condition B are met. If the load combinations of ACI 318 Appendix C are used, refer to Section D4.5 to determine the appropriate value of  $\phi$ .

5. Data for 5%" anchor is only valid for THDB62 series.

6. For sand-lightweight concrete, in lieu of ACI 318 Section D.3.4, modify the value of concrete breakout strength by 0.6. All-lightweight concrete is beyond the scope of this table.

### Titen HD® Tension and Shear Strength Design Data for Normal-Weight or Sand-Lightweight Concrete over Metal Deck<sup>1,2,6</sup>



See page 13 for an explanation of the oad table icons

				Lowe	r Flute	Uppe	r Flute	
Characteristic	Symbol	Units	Nomi	nal Anchor	Diameter	Nom. Anch. Diameter (inch		
			3/8 1/2		/2	3⁄8	1/2	
Embedment Depth	h <sub>nom</sub>	in.	1 7⁄8	21⁄2	2	31⁄2	1 7⁄8	2
Effective Embedment Depth	h <sub>ef</sub>	in.	1.23	1.77	1.29	2.56	1.23	1.29
Pullout Resistance, concrete on metal deck (cracked) <sup>3,4</sup>	N <sub>p,deck,cr</sub>	lbs.	375	870	905	2,040	500	1,700
Pullout Resistance, concrete on metal deck (uncracked) <sup>3,4</sup>	N <sub>p,deck,uncr</sub>	lbs.	825	1,905	1,295	2,910	1,095	2,430
Steel Strength in Shear, concrete on metal deck <sup>5</sup>	Vsa,deck	lbs.	2,240	2,395	2,435	4,430	4,180	7,145
Steel Strength in Shear, Seismic	Vsa, deck, eq	lbs.	1,434	1,533	1,556	2,846	2,676	4,591

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318 Appendix D, except as modified below.

2. Concrete compressive strength shall be 3,000 psi minimum. The characteristic pullout resistance for greater compressive strengths shall be increased by multiplying the tabular value by (f'c,specified / 3,000 psi)1/2.

3. For anchors installed in the soffit of sand-lightweight or normal-weight concrete over metal deck floor and roof assemblies, as shown in Figure A, calculation of the concrete breakout strength may be omitted.

4. In accordance with ACI 318 Section D.5.3.2, the nominal pullout strength in cracked concrete for anchors installed in the soffit of sand-lightweight or normal-weight-concrete-over-metal-deck floor and roof assemblies  $N_{\text{pn,deck,cr}}$  shall be substituted for  $N_{\text{pn,cr}}$ . Where analysis indicates no cracking at service loads, the normal pullout strength in uncracked concrete  $N_{\text{pn,deck,uncr}}$  shall be substituted for N<sub>pn,uncr</sub>

5. In accordance with ACI 318 Section D.6.1.2 (c), the shear strength for anchors installed in the soffit of sand-lightweight or normal-weight concrete over metal deck floor and roof assemblies Vsa.deck and Vsa, deck,eq shall be substituted for Vsa.

6. Minimum distance to edge of panel is 2hef

7. The minimum anchor spacing along the flute must be the greater of 3hef or 1.5 times the flute width.

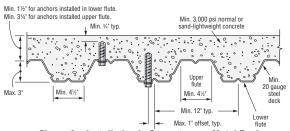


Figure A – Installation in Concrete over Metal Deck

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SIMPSON
Strong-Tie
®

### **Tension Loads in Normal-Weight Concrete**

Size	Drill	Embed.	Critical	Critical				Tension Load			
in. (mm)	Bit Dia.	Depth in.	Edge Dist.	Spacing Dist.		f' <sub>c</sub> ≥ 2000 ps 8 MPa) Cono		$f'_c \ge 3000 \text{ psi}$ (20.7 MPa) Concrete		f' <sub>c</sub> ≥ 4000 ps .6 MPa) Conc	
	in.	(mm)	in. (mm)	in. (mm)	Ultimate Ibs. (kN)	Std. Dev. Ibs. (kN)	Allowable Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Std. Dev. Ibs. (kN)	Allowable Ibs. (kN)
3/8	3/8	<b>2¾</b> (70)	3	6	<b>4,297</b> (19.1)	—	<b>1,075</b> (4.8)	<b>1,315</b> (5.8)	<b>6,204</b> (27.6)	_	<b>1,550</b> (6.9)
(9.5)	78	<b>3¾</b> (95)	(76)	(152)	<b>7,087</b> (31.5)	<b>347</b> (1.5)	<b>1,770</b> (7.9)	<b>2,115</b> (9.4)	<b>9,820</b> (43.7)	<b>1,434</b> (6.4)	<b>2,455</b> (10.9)
		<b>2¾</b> (70)			<b>4,610</b> (20.5)	—	<b>1,155</b> (5.1)	<b>1,400</b> (6.2)	<b>6,580</b> (29.3)	—	<b>1,645</b> (7.3)
<b>1⁄2</b> (12.7)	1⁄2	<b>35%</b> (92)	<b>4</b> (102)	<b>8</b> (203)	<b>7,413</b> (33.0)	<b>412</b> (1.8)	<b>1,855</b> (8.3)	<b>2,270</b> (10.1)	<b>10,742</b> (47.8)	<b>600</b> (2.7)	<b>2,685</b> (11.9)
		<b>5¾</b> (146)			<b>10,278</b> (45.7)	<b>297</b> (1.3)	<b>2,570</b> (11.4)	<b>3,240</b> (14.4)	<b>15,640</b> (69.6)	<b>2,341</b> (10.4)	<b>3,910</b> (17.4)
		<b>2¾</b> (70)			<b>4,610</b> (20.5)		<b>1,155</b> (5.1)	<b>1,400</b> (6.2)	<b>6,580</b> (29.3)	_	<b>1,645</b> (7.3)
<b>5⁄8</b> (15.9)	5⁄8	<b>41/8</b> (105)	<b>5</b> (127)	<b>10</b> (254)	<b>8,742</b> (38.9)	<b>615</b> (2.7)	<b>2,185</b> (9.7)	<b>2,630</b> (11.7)	<b>12,286</b> (54.7)	<b>1,604</b> (7.1)	<b>3,070</b> (13.7)
		<b>5¾</b> (146)			<b>12,953</b> (57.6)	<b>1,764</b> (7.8)	<b>3,240</b> (14.4)	<b>3,955</b> (17.6)	<b>18,680</b> (83.1)	_	<b>4,670</b> (20.8)
		<b>2¾</b> (70)			<b>4,674</b> (20.8)	—	<b>1,170</b> (5.2)	<b>1,405</b> (6.3)	<b>6,580</b> (29.3)	—	<b>1,645</b> (7.3)
<b>3⁄4</b> (19.1)	3⁄4	<b>45%</b> (117)	<b>6</b> (152)	<b>12</b> (305)	<b>10,340</b> (46.0)	<b>1,096</b> (4.9)	<b>2,585</b> (11.5)	<b>3,470</b> (15.4)	<b>17,426</b> (77.5)	<b>1,591</b> (7.1)	<b>4,355</b> (19.4)
		<b>5¾</b> (146)			<b>13,765</b> (61.2)	<b>1,016</b> (4.5)	<b>3,440</b> (15.3)	<b>4,055</b> (18.0)	<b>18,680</b> (83.1)	<b>1,743</b> (7.8)	<b>4,670</b> (20.8)

See Notes Below

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\*See page 13 for an explanation of the load table icons

Size	Drill	Embed.	Critical	Critical				Shear Load			
in. (mm)	Bit Dia.	Depth in.	Edge Dist.	Spacing Dist.	f' <sub>c</sub> ≥ 2000 psi (13.8 MPa) Concrete			f' <sub>c</sub> ≥ 3000 psi (20.7 MPa) Concrete	f' <sub>c</sub> ≥ 4000 psi (27.6 MPa) Concrete		
	in.	(mm)	in. (mm)	in. (mm)	Ultimate Ibs. (kN)	Std. Dev. Ibs. (kN)	Allowable lbs. (kN)	Allowable Ibs. (kN)	Ultimate Ibs. (kN)	Std. Dev. Ibs. (kN)	Allowable lbs. (kN)
3/8	3/8	<b>2¾</b> (70)	4 1/2	6	<b>6,353</b> (28.3)	_	<b>1,585</b> (7.1)	<b>1,665</b> (7.4)	—	_	<b>1,740</b> (7.7)
(9.5)	78	<b>3¾</b> (95)	(114)	(152)	<b>6,377</b> (28.4)	<b>1,006</b> (4.5)	<b>1,595</b> (7.1)	<b>1,670</b> (7.4)	—	_	<b>1,740</b> (7.7)
		<b>2¾</b> (70)			<b>6,435</b> (28.6)	_	<b>1,605</b> (7.1)	<b>2,050</b> (9.1)	<b>9,987</b> (44.4)	_	<b>2,495</b> (7.8)
<b>1⁄2</b> (12.7)	1⁄2	<b>35/8</b> (92)	<b>6</b> (152)	<b>8</b> (203)	<b>9,324</b> (41.5)	<b>1,285</b> (5.7)	<b>2,330</b> (10.4)	<b>2,795</b> (12.4)	<b>13,027</b> (57.9)	<b>597</b> (2.7)	<b>3,255</b> (14.5)
. ,		<b>5¾</b> (146)			<b>11,319</b> (50.3)	<b>1,245</b> (5.5)	<b>2,830</b> (12.6)	<b>3,045</b> (13.5)	_	_	<b>3,255</b> (14.5)
		<b>2<sup>3</sup>/4</b> (70)			<b>7,745</b> (34.5)	_	<b>1,940</b> (8.6)	<b>2,220</b> (9.9)	<b>9,987</b> (44.4)	_	<b>2,495</b> (11.1)
<b>5⁄8</b> (15.9)	5⁄8	<b>41/8</b> (105)	<b>7½</b> (191)	<b>10</b> (254)	<b>8,706</b> (38.7)	<b>1,830</b> (8.1)	<b>2,175</b> (9.7)	<b>3,415</b> (15.2)	<b>18,607</b> (82.8)	<b>1,650</b> (7.3)	<b>4,650</b> (20.7)
. ,		<b>5¾</b> (146)			<b>12,498</b> (55.6)	<b>2,227</b> (9.9)	<b>3,125</b> (13.9)	<b>3,890</b> (17.3)	—	_	<b>4,650</b> (20.7)
		<b>2<sup>3</sup>/4</b> (70)			<b>7,832</b> (34.8)	_	<b>1,960</b> (8.7)	<b>2,415</b> (10.7)	<b>11,460</b> (51.0)	—	<b>2,865</b> (12.7)
<b>3⁄4</b> 19.1)	3⁄4	<b>45%</b> (117)	<b>9</b> (229)	<b>12</b> (305)	<b>11,222</b> (49.9)	<b>2,900</b> (12.9)	<b>2,805</b> (12.5)	<b>4,490</b> (20.0)	<b>24,680</b> (109.8)	<b>2,368</b> (10.5)	<b>6,170</b> (27.4)
(		<b>5¾</b> (146)			<b>19,793</b> (88.0)	<b>3,547</b> (15.8)	<b>4,950</b> (22.0)	<b>5,560</b> (24.7)	<b>24,680</b> (109.8)	<b>795</b> (3.5)	<b>6,170</b> (27.4)

### Shear Loads in Normal-Weight Concrete

1. The allowable loads listed are based on a safety factor of 4.0.

2. Refer to allowable load-adjustment factors for spacing and edge distance on pages 119-120.

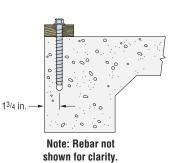
3. The minimum concrete thickness is 1 ½ times the embedment depth.
4. Tension and Shear loads for the Titen HD anchor may be combined using the elliptical

interaction equation (n=%). Allowable load may be interpolated for concrete compressive strengths between 2000 psi and 4000 psi.



Shear Loads in Normal-Weight Concrete, Load Applied Parallel to Concrete Edge

Size in.	Drill Bit Dia.	Embed. Depth	Minimum Edge	Minimum End	Minimum Spacing	Conci	ar Load Base rete Edge Dis	tance
(mm)	in.	in. (mm)	Dist. in. (mm)	Dist. in. (mm)	Dist. in. (mm)	f' <sub>c</sub> ≥ 2500 µ Ultimate Ibs. (kN)	osi (17.2 MPa Std. Dev. Ibs. (kN)	a) Concrete Allowable Ibs. (kN)
		<b>2¾</b> (70)				<b>4,660</b> (20.7)	<b>575</b> (2.6)	<b>1,165</b> (5.2)
1/2	1/	<b>31⁄4</b> (83)	1¾	8	8	_	_	<b>1,530</b> (6.8)
(12.7)	1⁄2	<b>3 ½</b> (89)	(45)	(203)	(203)	<b>6,840</b> (30.4)	<b>860</b> (3.8)	<b>1,710</b> (7.6)
		<b>4½</b> (114)				<b>7,800</b> (34.7)	<b>300</b> (1.3)	<b>1,950</b> (8.7)
		<b>2<sup>3</sup>⁄4</b> (70)				<b>4,820</b> (21.4)	<b>585</b> (2.6)	<b>1,205</b> (5.3)
<b>5⁄8</b> (15.9)	5⁄8	<b>31⁄4</b> (83)	<b>1¾</b> (45)	<b>10</b> (254)	<b>10</b> (254)	_	_	<b>1,580</b> (7.0)
. ,		<b>3 ½</b> (89)				<b>7,060</b> (31.4)	<b>1,284</b> (5.7)	<b>1,765</b> (7.9)



The allowable loads listed are based on a safety factor of 4.0.

 The minimum concrete thickness is 1 <sup>1</sup>/<sub>2</sub> times the embedment depth.

\*See page 13 for an explanation of the load table icons

The Titen HD<sup>®</sup> screw anchor may be used for sill plate applications. Use bearing plates as required by code. Refer to the appropriate code report or use Simpson Strong-Tie ACI 318 Anchor Selector<sup>™</sup> software for anchor design information.



Titen HD Screw Anchor U.S. Patent 5,674,035 & 6,623,228





Tensio	n Loa	ids in No	ormal-We	ight Co	oncret	e Stemw	all			
							Tensio	n Load		
Size in. (mm)	Drill Bit Dia.	Embed. Depth in.	Stemwall Width in.	Min. Edge Dist.	Min. End Dist.	f' <sub>c</sub> ≥ 250 (17.2 ľ Conci	MPa)	f' <sub>c</sub> ≥ 450 (31.0 ľ Conci	VIPa)	
	in.	(mm)	(mm)	in. (mm)	in. (mm)	Ultimate Ibs. (kN)	Allow. Ibs. (kN)	Ultimate Ibs. (kN)	Allow. Ibs. (kN)	
1/2	1/	10	6	1¾	<b>8</b> (203)	<b>15,420</b> (68.6)	<b>3,855</b> (17.1)	<b>20,300</b> (90.3)	<b>5,075</b> (22.6)	
(12.7)	1⁄2	(254)	(152)	(44)	<b>4</b> <sup>3</sup> / <sub>8</sub> (111)	<b>14,280</b> (63.5)	<b>3,570</b> (15.9)	<b>19,040</b> (84.7)	<b>4,760</b> (21.2)	
1. The al	1. The allowable loads are based 3. The minimum concrete thickness									

on a safety factor of 4.0. 2. The minimum anchor spacing is 15 inches.

(depth) is 12 inches. 4. Allowable loads may be interpolated for compressive strengths between 2,500 and 4,500 psi.

Tension Loads in Normal-Weight Concrete, Load Applied at 60° Angle to Horizontal for Tilt-Up Wall Braces

Size	Drill Bit	Embed.		Applied at 60 to Horizonta				
in. (mm)	Dia. in.	Depth in. (mm)	f' <sub>c</sub> ≥ 2500 psi (17.2 MPa) Concrete					
			Ultimate Ibs. (kN)	Std. Dev. Ibs. (kN)	Allow. Ibs. (kN)			
<b>5⁄8</b> (15.9)	5⁄8	<b>5</b> (127)	<b>13,420</b> (59.7)	<b>1,273</b> (5.7)	<b>3,355</b> (14.9)			
<b>3⁄4</b> (19.1)	3⁄4	<b>5</b> (127)	<b>15,180</b> (67.5)	<b>968</b> (4.3)	<b>3,795</b> (16.9)			

1. The allowable loads are based on a safety factor of 4.0.

2. Anchor must be installed into a concrete floor slab, footing, or deadman with sufficient area, weight, and strength to resist the anchorage load.

3. Titen HD® has been qualified for temporary outdoor use of up to 90 days through testing for this application.

### Tension and Shear Loads in Sand-Lightweight Concrete over Metal Deck

					Ins	tall in Concre	te (see Figure	A)	Install	through Meta	l Deck (see Fi	gure A)
Size in.	Drill Bit	Embed. Depth	Critical Edge	Critical Spacing	Tensio	n Load	Shear	<sup>,</sup> Load	Tensio	n Load	Shear	Load
(mm)	Dia. in.	in. (mm)	Dist.	Dist.	f' <sub>c</sub> ≥ 3000 ps Lightweigh	i (20.7 MPa) It Concrete	f' <sub>c</sub> ≥ 3000 ps Lightweigh	i (20.7 MPa) nt Concrete	f' <sub>c</sub> ≥ 3000 ps Lightweigl	i (20.7 MPa) nt Concrete		i (20.7 MPa) It Concrete
			(mm)	(mm)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Allowable lbs. (kN)
3/8	8/	<b>2¾</b> (70)	6	6	<b>2,560</b> (11.4)	<b>640</b> (2.8)	<b>4,240</b> (18.9)	<b>1,060</b> (4.7)	_	_	_	
(9.5)		3	(152)	(152)	_	_	_	_	<b>5,420</b> (24.1)	<b>1,355</b> (6.0)	<b>4,100</b> (18.2)	<b>1,025</b> (4.6)
1/2	1/2	<b>2¾</b> (70)	8	8	<b>3,040</b> (13.5)	<b>760</b> (3.4)	<b>6,380</b> (28.4)	<b>1,595</b> (7.1)	_	_	_	—
(12.7)	72	<b>4</b> (102)	(203)	(203)	_	_	_	_	<b>7,020</b> (31.2)	<b>1,755</b> (7.8)	<b>6,840</b> (30.4)	<b>1,710</b> (7.6)
5⁄8	5/-	<b>2¾</b> (70)	10	10	<b>3,100</b> (13.8)	<b>775</b> (3.4)	<b>6,380</b> (28.4)	<b>1,595</b> (7.1)	_	_	_	_
<b>%8 5⁄8</b> (15.9)		(254)	(254)		_			<b>8,940</b> (39.8)	<b>2,235</b> (9.9)	<b>10,700</b> (47.6)	<b>2,675</b> (11.9)	

1. The allowable loads listed are based on a safety factor of 4.0. 3. Anchors may be installed off-center in the 2. Allowable loads for anchors installed in the lower flute of the steel deck are for flutes with a trapezoidal profile with a depth of 3 inches, and a width varying from 41/2 inches at the bottom to 71/2 inches at the top. The spacing of the flutes is 12 inches. The metal deck must be minimum 20-gauge with a minimum yield strength of 38 ksi and minimum ultimate strength of 45 ksi.

lower flute (up to 11/2" from the edge of the lower flute) without a load reduction.

4. 100% of the allowable load is permitted at critical edge distance and critical spacing. Testing at smaller edge distances and spacings has not been performed.

\*See page 13 for an explanation of the load table icons

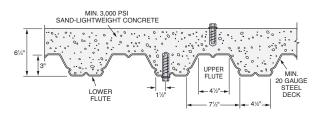


Figure A - Titen HD® screw anchor installed in the top and bottom of a structural sand-lightweight-concrete and metal-deck assembly

simpsoi Strono-

Tension and Shear Loads in 8-inch Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU



See page 13 for an explanation of the load table icons

Size in.	Drill Bit	Min. Embed.	Critical Edge	Critical End	Critical Spacing					
(mm)	Dia.	Depth	Dist.	Dist.	Dist.	Tensio	n Load	Shear Load		
	in.	in. (mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate Allowable Ibs. (kN) Ibs. (kN)		Ultimate Ibs. (kN)	Allowable Ibs. (kN)	
		An	chor Insta	alled in th	e Face of	the CMU Wa	ll (See Figure	e 1)		
<b>3⁄8</b> (9.5)	3⁄8	<b>2¾</b> (70)	<b>12</b> (305)	<b>12</b> (305)	<b>6</b> (152)	<b>2,390</b> (10.6)	<b>480</b> (2.1)	<b>4,340</b> (19.3)	<b>870</b> (3.9)	
<b>1/2</b> (12.7)	1⁄2	<b>3 ½</b> (89)	<b>12</b> (305)	<b>12</b> (305)	<b>8</b> (203)	<b>3,440</b> (15.3)	<b>690</b> (3.1)	<b>6,920</b> (30.8)	<b>1,385</b> (6.2)	
<b>5⁄8</b> (15.9)	5⁄8	<b>4½</b> (114)	<b>12</b> (305)	<b>12</b> (305)	<b>10</b> (254)	<b>5,300</b> (23.6)	<b>1,060</b> (4.7)	<b>10,420</b> (46.4)	<b>2,085</b> (9.3)	
<b>3⁄4</b> (19.1)	3⁄4	<b>5½</b> (140)	<b>12</b> (305)	<b>12</b> (305)	<b>12</b> (305)	<b>7,990</b> (35.5)	<b>1,600</b> (7.1)	<b>15,000</b> (66.7)	<b>3,000</b> (13.3)	

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

2. Values for 8-inch wide, lightweight, medium-weight and normal-weight concrete masonry units.

3. The masonry units must be fully grouted.

The minimum specified compressive strength of masonry, f<sup>'</sup><sub>m</sub>, at 28 days is 1,500 psi.
 Embedment depth is measured from the outside face of the concrete masonry unit.

6. Allowable loads may be increased 33 1/3% for short-term loading due to wind or seismic forces where permitted by code.

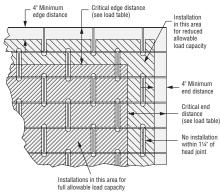
7. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.

8. Refer to allowable load-adjustment factors for spacing and edge distance on page 121.

#### Tension and Shear Loads in 8-inch Lightweight, Medium-Weight and Normal-Weight Hollow CMU

moura		igni ana				01110			J
Size in.	Drill Bit	Embed. Depth <sup>4</sup>	Min. Edge	Min. End	8-ind	h Hollow Cl on CMU	MU Loads I Strength	Based	1. The tabulated allowable loads are based on a safety factor of
(mm)	Dia.	in. (mm)	Dist.	Dist.	Tensio	n Load	Shea	r Load	<ol> <li>5.0 for installations under the IBC and IRC.</li> <li>2. Values for 8-inch wide, lightweight, medium-weight</li> </ol>
		()	(mm)	(mm)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Allowable Ibs. (kN)	and normal-weight concrete masonry units. 3. The minimum specified compressive strength of masonry, f' <sub>m</sub> , at 28 days <b>Figu</b>
		Anc	hor Inst	alled in	Face Shell	(See Figure	2)		is 1,500 psi.
<b>3⁄8</b> (9.5)	3⁄8	<b>1¾</b> (44)	<b>4</b> (102)	<b>45%</b> (117)	<b>720</b> (3.2)	<b>145</b> (0.6)	<b>1,240</b> (5.5)	<b>250</b> (1.1)	4. Embedment depth is measured from the outside face of the concrete masonry unit and is based on the
<b>1⁄2</b> (12.7)	1⁄2	<b>1¾</b> (44)	<b>4</b> (102)	<b>45%</b> (117)	<b>760</b> (3.4)	<b>150</b> (0.7)	<b>1,240</b> (5.5)	<b>250</b> (1.1)	1/2" through 1 ¼" thick face shell. 5. Allowable loads may not be
<b>5⁄8</b> (15.9)	5⁄8	<b>1¾</b> (44)	<b>4</b> (102)	<b>45%</b> (117)	<b>800</b> (3.6)	<b>160</b> (0.7)	<b>1,240</b> (5.5)	<b>250</b> (1.1)	increased for short-term loading due to wind or seismic forces. CMU wall design must satisfy
<b>3⁄4</b> (19.1)	3⁄4	<b>1¾</b> (44)	<b>4</b> (102)	<b>45%</b> (117)	<b>880</b> (3.9)	<b>175</b> (0.8)	<b>1,240</b> (5.5)	<b>250</b> (1.1)	<ul> <li>applicable design standards and be capable of withstanding applied loads.</li> <li>6. Do not use impact wrenches to install in hollow CMU.</li> <li>7. Set drill to rotation-only mode when drilling into hollow CMU.</li> </ul>

Figure 1



Shaded Area = Placement for Full and Reduced Allowable Load **Capacity in Grout-Filled CMU** 

- 2. Values for 8-inch wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry, f'm, at 28 days is 1.500 psi.
- 4. Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional 1/2" through 1 1/4" thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces. CMU wall design must satisfy





Figure 2

### Tension and Shear Loads in 8-inch Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall

Size	Drill	Embed.	Min.	Min.	Critical	8-inch G	rout-Filled C	MU Allowal	ble Loads Ba	ised on CMU	Strength	
in. (mm)	Bit Dia.	Depth in.	Edge Dist.	End Dist.	Spacing Dist.	Ten	sion	Shear Per	rp. to Edge	Shear Para	llel to Edge	
	in.	(mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	Ultimate Ibs. (kN)	Allowable Ibs. (kN)	Ultimate Ibs. (kN)	Allowable lbs. (kN)	
			Anch	or Insta	lled in Cel	l Opening o	r Web (Top c	of Wall) (Se	e Figure 3)			
<b>1⁄2</b> (12.7)	1⁄2	<b>4½</b> (114)	<b>1 <sup>3</sup>⁄4</b> (44.5)	<b>8</b> (203)	<b>8</b> (203)	<b>2,860</b> (12.7)	<b>570</b> (2.5)	<b>800</b> (3.6)	<b>160</b> (0.7)	<b>2,920</b> (13.0)	<b>585</b> (2.6)	
<b>5⁄8</b> (15.9)	5⁄8	<b>4½</b> (114)	<b>1 <sup>3</sup>⁄4</b> (44.5)	<b>10</b> (254)	<b>10</b> (254)	<b>2,860</b> (12.7)	<b>570</b> (2.5)	<b>800</b> (3.6)	<b>160</b> (0.7)	<b>3,380</b> (15.0)	<b>675</b> (3.0)	

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

2. Values are for 8-inch wide, lightweight, medium-weight and normal-weight concrete masonry units.

3. The masonry units must be fully grouted.

The minimum specified compressive strength of masonry, f<sup>'</sup><sub>m</sub>, at 28 days is 1,500 psi.
 Allowable loads may be increased 33 ¼% for short-term loading due to wind or seismic forces where permitted by code.

6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.

7. Loads are based on anchor installed in either the web or grout-filled cell opening in the top of wall.

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**Mechanical Anchors** 

Figure 3 1¾" Edge 000 . D 0 End 0 0 <u>`</u>0 ٥Ô



### Titen HD<sup>®</sup> Technical Information

### Load-Adjustment Factors for Titen HD® Anchors in Normal-Weight Concrete: Edge Distance, Tension and Shear Loads

#### How to use these charts:

- 1. The following tables are for reduced edge distance.
- 2. Locate the anchor size to be used for either a tension
- and/or shear load application.
- 3. Locate the anchor embedment (E) used for either a tension and/or shear load application.

- 4. Locate the edge distance ( $C_{act}$ ) at which the anchor is to be installed. 5. The load adjustment factor ( $f_c$ ) is the intersection of the row and column. 6. Multiply the allowable load by the applicable load adjustment factor(s).

7. Reduction factors for multiple edges are multiplied together.

	Dia.	3	8		1/2			5/8			3/4	
Edge	E	23/4	3¾	23⁄4	35/8	5 <sup>3</sup> ⁄4	23⁄4	41/8	5¾	23⁄4	4 5/8	5 <sup>3</sup> ⁄4
Dist.	C <sub>cr</sub>	3	3	4	4	4	5	5	5	6	6	6
C <sub>act</sub> (in.)	Cmin	1¾	1¾	1¾	1¾	1¾	1¾	1¾	1¾	1 3⁄4	1¾	1¾
()	<b>f</b> cmin	0.83	0.73	0.67	0.57	0.73	0.67	0.57	0.59	0.67	0.48	0.58
1¾		0.83	0.73	0.67	0.57	0.73	0.67	0.57	0.59	0.67	0.48	0.58
2		0.86	0.78	0.71	0.62	0.76	0.70	0.60	0.62	0.69	0.51	0.60
21⁄4		0.90	0.84	0.74	0.67	0.79	0.72	0.64	0.65	0.71	0.54	0.63
21⁄2		0.93	0.89	0.78	0.71	0.82	0.75	0.67	0.68	0.73	0.57	0.65
2 ¾		0.97	0.95	0.82	0.76	0.85	0.77	0.70	0.72	0.75	0.60	0.68
3		1.00	1.00	0.85	0.81	0.88	0.80	0.74	0.75	0.77	0.63	0.70
31⁄4				0.89	0.86	0.91	0.82	0.77	0.78	0.79	0.66	0.73
31⁄2				0.93	0.90	0.94	0.85	0.80	0.81	0.81	0.69	0.75
3¾				0.96	0.95	0.97	0.87	0.83	0.84	0.83	0.72	0.78
4				1.00	1.00	1.00	0.90	0.87	0.87	0.84	0.76	0.80
4 1⁄4							0.92	0.90	0.91	0.86	0.79	0.83
41⁄2							0.95	0.93	0.94	0.88	0.82	0.85
4 <b>¾</b>							0.97	0.97	0.97	0.90	0.85	0.88
5							1.00	1.00	1.00	0.92	0.88	0.90
51⁄4										0.94	0.91	0.93
5½										0.96	0.94	0.95
5 <b>¾</b>										0.98	0.97	0.98
6										1.00	1.00	1.00

See Notes Below

### Edgo Distanco Shoar (f.)

	Dia.	3	8		1/2			5⁄8			3⁄4	
Edge	Е	2¾	3¾	2 3⁄4	3 5⁄8	5 <sup>3</sup> ⁄4	2 3⁄4	4 1/8	5 <sup>3</sup> ⁄4	2 3⁄4	4 5⁄8	<b>5</b> <sup>3</sup> ⁄4
Dist.	C <sub>cr</sub>	4 1/2	4 1/2	6	6	6	71⁄2	7 1⁄2	71⁄2	9	9	9
C <sub>act</sub> (in.)	C <sub>min</sub>	1¾	1¾	1 3⁄4	1¾	1 3⁄4	1 3⁄4	1¾	1 3⁄4	1 3⁄4	1¾	1 3⁄4
()	<b>f</b> cmin	0.25	0.24	0.25	0.20	0.17	0.19	0.16	0.19	0.19	0.14	0.13
1 3⁄4		0.25	0.24	0.25	0.20	0.17	0.19	0.16	0.19	0.19	0.14	0.13
2		0.32	0.31	0.29	0.25	0.22	0.23	0.20	0.23	0.22	0.17	0.16
21⁄2		0.45	0.45	0.38	0.34	0.32	0.30	0.27	0.30	0.27	0.23	0.22
3		0.59	0.59	0.47	0.44	0.41	0.37	0.34	0.37	0.33	0.29	0.28
31⁄2		0.73	0.72	0.56	0.53	0.51	0.44	0.42	0.44	0.39	0.35	0.34
4		0.86	0.86	0.65	0.62	0.61	0.51	0.49	0.51	0.44	0.41	0.40
4 1⁄2		1.00	1.00	0.74	0.72	0.71	0.58	0.56	0.58	0.50	0.47	0.46
5				0.82	0.81	0.80	0.65	0.63	0.65	0.55	0.53	0.52
5½				0.91	0.91	0.90	0.72	0.71	0.72	0.61	0.58	0.58
6				1.00	1.00	1.00	0.79	0.78	0.79	0.66	0.64	0.64
61⁄2							0.86	0.85	0.86	0.72	0.70	0.70
7							0.93	0.93	0.93	0.78	0.76	0.76
7 <b>1⁄2</b>							1.00	1.00	1.00	0.83	0.82	0.82
8										0.89	0.88	0.88
81⁄2										0.94	0.94	0.94
9										1.00	1.00	1.00

\*See page 13 for an explanation of the load table icons

The tabled adjustment values ( $f_{\rm c})$  have been calculated using the following information:

- 1. E = Embedment depth (inches).
- 2. C<sub>act</sub> = actual edge distance at which anchor is installed (inches).
- 3. C<sub>cr</sub> = critical edge distance for 100% load (inches).
- 4. Cmin = minimum edge distance for reduced load (inches).
- 5. fc = percent of allowable load at actual edge distance.
- 6. fccr = percentage of allowable load at critical edge distance.  $f_{ccr}$  is always = 1.00.
- 7.  $f_{cmin}$  = percent of allowable load at minimum edge distance. 8.  $f_c = f_{cmin} + [(1 f_{cmin})]$
- (C<sub>act</sub> C<sub>min</sub>) / (C<sub>cr</sub> C<sub>min</sub>)].



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## Titen HD<sup>®</sup> Technical Information

### Load-Adjustment Factors for Titen HD® Anchors in Normal-Weight Concrete: Spacing, Tension and Shear Loads

#### How to use these charts:

Spacing Tension (fs)

- 1. The following tables are for reduced spacing.
- 2. Locate the anchor size to be used for either a tension
- and/or shear load application.

- 4. Locate the spacing  $(S_{act})$  at which the anchor is to be installed. 5. The load adjustment factor  $(f_s)$  is the intersection of the row and column. 6. Multiply the allowable load by the applicable load adjustment factor(s).
- 7. Reduction factors for multiple spacings are multiplied together.
- 3. Locate the anchor embedment (E) used for either a tension and/or shear load application.
- Q Q

1.00

1.00

1.00

	5	(10)								2008		in the second
	Dia.	3	8		1/2			5⁄8			3⁄4	
•	E	2¾	3¾	23⁄4	3 5⁄8	5 <sup>3</sup> ⁄4	2 3⁄4	4 1/8	5 <sup>3</sup> ⁄4	2 3⁄4	4 5⁄8	5 <sup>3</sup> ⁄4
S <sub>act</sub> (in.)	Scr	6	6	8	8	8	10	10	10	12	12	12
()	Smin	1 1⁄2	11/2	2	2	2	21⁄2	2 1⁄2	21/2	3	3	3
	<b>f</b> smin	0.66	0.56	0.72	0.63	0.76	0.79	0.69	0.73	0.80	0.70	0.72
1 1⁄2		0.66	0.56									
2		0.70	0.61	0.72	0.63	0.76						
21⁄2		0.74	0.66	0.74	0.66	0.78	0.79	0.69	0.73			
3		0.77	0.71	0.77	0.69	0.80	0.80	0.71	0.75	0.80	0.70	0.72
4		0.85	0.80	0.81	0.75	0.84	0.83	0.75	0.78	0.82	0.73	0.75
5		0.92	0.90	0.86	0.82	0.88	0.86	0.79	0.82	0.84	0.77	0.78
6		1.00	1.00	0.91	0.88	0.92	0.89	0.83	0.86	0.87	0.80	0.81
7				0.95	0.94	0.96	0.92	0.88	0.89	0.89	0.83	0.84
8				1.00	1.00	1.00	0.94	0.92	0.93	0.91	0.87	0.88
9							0.97	0.96	0.96	0.93	0.90	0.91
10							1.00	1.00	1.00	0.96	0.93	0.94
11										0.98	0.97	0.97

See page 13 for an explanation of the load table icons

1

	Dia.	3	8		1/2			5⁄8			3⁄4		
•	E	2¾	3¾	23⁄4	3 5⁄8	5¾	2¾	4 1/8	5¾	2¾	4 5⁄8	5 <sup>3</sup> ⁄4	1
S <sub>act</sub> (in.)	Scr	6	6	8	8	8	10	10	10	12	12	12	The t have
()	Smin	1 1⁄2	11⁄2	2	2	2	21⁄2	2 1⁄2	21/2	3	3	3	follo
	<b>f</b> <sub>smin</sub>	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	1.E =
11⁄2		0.77	0.77										2. Sa wł
2		0.80	0.80	0.77	0.77	0.77							(in
21⁄2		0.82	0.82	0.79	0.79	0.79	0.77	0.77	0.77				3. S <sub>cr</sub>
3		0.85	0.85	0.81	0.81	0.81	0.79	0.79	0.79	0.77	0.77	0.77	10 4. Sm
4		0.90	0.90	0.85	0.85	0.85	0.82	0.82	0.82	0.80	0.80	0.80	for
5		0.95	0.95	0.89	0.89	0.89	0.85	0.85	0.85	0.82	0.82	0.82	5. f <sub>s</sub> :
6		1.00	1.00	0.92	0.92	0.92	0.88	0.88	0.88	0.85	0.85	0.85	alle sp
7				0.96	0.96	0.96	0.91	0.91	0.91	0.87	0.87	0.87	6. f <sub>scr</sub>
8				1.00	1.00	1.00	0.94	0.94	0.94	0.90	0.90	0.90	allo dis
9							0.97	0.97	0.97	0.92	0.92	0.92	7. f <sub>sm</sub>
10							1.00	1.00	1.00	0.95	0.95	0.95	allo
11										0.97	0.97	0.97	spa 8. f <sub>s</sub> =
12										1.00	1.00	1.00	(Sa

stment values (f<sub>s</sub>) lated using the

- nation: ent depth (inches).
- spacing distance at rs are installed
- spacing distance for
- nches). um spacing distance
- oad (inches). ent factor for
- d at actual ince.
- ent factor for d at critical spacing
- is always = 1.00. ment factor for d at minimum nce.
  - f<sub>smin</sub>)
  - (S<sub>cr</sub> S<sub>min</sub>)].

12 See Notes Below



These tables are not for use

with SD design methods

### Titen HD<sup>®</sup> Technical Information

#### Load-Adjustment Factors for Titen HD® Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads

#### How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or
- shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.

Edge or Tension		tance				*
	Dia.	3⁄8	1⁄2	5⁄8	3⁄4	
0	E	23⁄4	31⁄2	4 1/2	51⁄2	
C <sub>act</sub> (in.)	Ccr	12	12	12	12	
()	C <sub>min</sub>	4	4	4	4	
	f <sub>cmin</sub>	1.00	1.00	0.83	0.66	
4		1.00	1.00	0.83	0.66	
6		1.00	1.00	0.87	0.75	
8		1.00	1.00	0.92	0.83	
10		1.00	1.00	0.96	0.92	
12		1.00	1.00	1.00	1.00	

See Notes Below

Edge or End Distance Shear (fc)			
Shear Load Perpendicular to Edge or End (Directed Towards Edge or End)		(	<b></b> *
End (Directed Towards Edge or End)	र विद्य		

						u
	Dia.	3⁄8	1⁄2	5⁄8	3⁄4	
•	E	2¾	31⁄2	4 1/2	5½	
C <sub>act</sub> (in.)	C <sub>cr</sub>	12	12	12	12	
()	Cmin	4	4	4	4	
	<b>f</b> cmin	0.58	0.38	0.30	0.21	
4		0.58	0.38	0.30	0.21	
6		0.69	0.54	0.48	0.41	
8		0.79	0.69	0.65	0.61	
10		0.90	0.85	0.83	0.80	
12		1.00	1.00	1.00	1.00	

1. E = Embedment depth (inches).

2. Cact = actual end or edge distance at which anchor is installed (inches).

- 3. C<sub>cr</sub> = critical end or edge distance for 100% load (inches).
- 4. C<sub>min</sub> = minimum end or edge distance for reduced load (inches).
- 5.  $f_c$  = adjustment factor for allowable load at actual end or edge distance.
- 6.  $f_{cor}$  = adjustment factor for allowable load at critical end or edge distance.  $f_{cor}$  is always = 1.00.

7. f<sub>cmin</sub> = adjustment factor for allowable load at minimum end or edge distance.

8.  $f_c = f_{cmin} + [(1 - f_{cmin}) (C_{act} - C_{min}) / (C_{cr} - C_{min})].$ 

Chaoina	Tension	(4)

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Spacing Tension (f <sub>s</sub> )					
	Dia.	3⁄8	1/2	5⁄8	3⁄4
	E	2¾	31⁄2	4 1/2	5 ½
S <sub>act</sub> (in.)	Scr	6	8	10	12
()	Smin	3	4	5	6
	<b>f</b> <sub>smin</sub>	0.87	0.69	0.59	0.50
3		0.87			
4		0.91	0.69		
5		0.96	0.77	0.59	
6		1.00	0.85	0.67	0.50
8			1.00	0.84	0.67
10				1.00	0.83
12					1.00

1. E = Embedment depth (inches).

2. S<sub>act</sub> = actual spacing distance at which anchors are installed (inches).

- 3. S<sub>cr</sub> = critical spacing distance for 100% load (inches).
- $\rm A. S_{min}$  = minimum spacing distance for reductions). 4. S<sub>min</sub> = minimum spacing distance for reduced load (inches). 5. f<sub>s</sub> = adjustment factor for allowable load at actual spacing distance.

6. fscr = adjustment factor for allowable load at critical spacing distance. fscr is always = 1.00.

7.  $f_{smin}$  = adjustment factor for allowable load at minimum spacing distance.



Strong-Tie

- 4. Locate the edge distance ( $C_{act}$ ) or spacing ( $S_{act}$ ) at which the anchor is to be installed.
- The load adjustment factor ( $f_{\rm C} \mbox{ or } f_{\rm S})$  is the intersection of the row and column. 5.
- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

Edge and End Distance Shear (f<sub>c</sub>)

	Edge and End Distance Shear (f <sub>c</sub> ) (Constrained by the second sec						
	Dia.	3⁄8	1⁄2	5⁄8	3⁄4		
•	E	23⁄4	3 1/2	4 ½	5 ½		
C <sub>act</sub> (in.)	Ccr	12	12	12	12		
()	C <sub>min</sub>	4	4	4	4		
	<b>f</b> <sub>cmin</sub>	0.77	0.48	0.46	0.44		
4		0.77	0.48	0.46	0.44		
6		0.83	0.61	0.60	0.58		
8		0.89	0.74	0.73	0.72		
10		0.94	0.87	0.87	0.86		
				1.00	1.00		

See Notes Below

### Edge or End Distance Shear (f<sub>c</sub>) Shear Load Perpendicular to Edge or End (Directed Away From Edge or End)

	Dia.	3⁄8	1⁄2	5⁄8	3⁄4
•	E	2¾	31⁄2	4 1/2	5½
C <sub>act</sub> (in.)	C <sub>cr</sub>	12	12	12	12
()	C <sub>min</sub>	4	4	4	4
	f <sub>cmin</sub>	0.89	0.79	0.58	0.38
4		0.89	0.79	0.58	0.38
6		0.92	0.84	0.69	0.54
8		0.95	0.90	0.79	0.69
10		0.97	0.95	0.90	0.85
12		1.00	1.00	1.00	1.00

\*See page 13 for an explanation of the load table icons

	Dia.	3⁄8	1/2	5⁄8	3⁄4
•	E	2¾	31⁄2	4 1/2	51⁄2
S <sub>act</sub> (in.)	t S <sub>cr</sub>	6	8	10	12
()	Smin	3	4	5	6
	<b>f</b> smin	0.62	0.62	0.62	0.62
3		0.62			
4		0.75	0.62		
5		0.87	0.72	0.62	
6		1.00	0.81	0.70	0.62
8			1.00	0.85	0.75
10				1.00	0.87
12					1.00